# LONGITUDINAL VENTILATION SYSTEM FOR A LONG ROADTUNNEL: OPTIMAL DESIGN WITH BATTERIES OF JET FANS AND CHALLENGES TO OVERCOME EXTREME FOGGY WEATHER CONDITION 

Sunit Kanti Dhar<br>Freelance Consulting Engineer - Tunnel Ventilation \& Fire Safety and E\&M Services, IN

DOI 10.3217/978-3-85125-996-4-38 (CC BY-NC 4.0)
This CC license does not apply to third party material and content noted otherwise.


#### Abstract

An optimal design solution for a long road-tunnel mechanical ventilation system with an application of batteries of jet fans at a location where extreme foggy weather conditions prevails for a third of the year. The main purpose is to make it practically feasible, by applying the concept of longitudinal ventilation system, as well as optimizing the design to keep the vehicular pollutants under control and fully functional to high standard of international design parameters and criterion, even during heavy fog and rainfall, for normal mode ventilation, particularly during congestion and slow-moving traffic, as well as to cater to an effective smoke management at any probable locations of the fire scenarios.

The design will be innovative for application for such a long road-tunnel with heavy traffic profile, and very practical for application that will harmonize with good engineering practices and international codes and guidelines with an objective to achieve highest standards of safety criteria, both during normal and emergency situations.


Keywords: Design, Foggy, Longitudinal, Road-Tunnel, Ventilation, Extinction Coefficient Fire, Smoke.

## 1. INTRODUCTION

The purpose of this paper is to identify and address the challenges faced to overcome the extreme foggy weather condition that prevails during rainy climatic conditions for a third of the year in the location of this long road tunnel at a medium altitude hilly terrain.

The longitudinal mechanical ventilation system design philosophy with batteries of jet fans being adopted for this 8.67 km long unidirectional rural parallel twin tube road tunnel with four traffic lanes in each tunnel-bore, because of the terrain and inaccessibility for an intermediate ventilation shaft(s), as well as for the protection of the wildlife and the forest, specifically in the region where these twin tunnels are passing through.

The main purpose is to make it practically feasible, as well as optimizing the design to keep the vehicular pollutants under control and fully functional to high standard of international design parameters and criterion, even during heavy fog and rainfall, for normal mode ventilation, particularly during congestion and slow-moving traffic, as well as to cater to an effective smoke management at any probable locations of the fire incident.

The detailed design of the ventilation system has been independently carried out with both PIARC [1] and RVS [2] guidelines for the vehicle emission and fresh air demand for ventilation calculations. However, in this paper the design with PIARC the methodology, design aspects and details with calculations are discussed here in this paper.

## 2. DESIGN

### 2.1 Design parameters under consideration for this case study

### 2.1.1 Physical design parameters and geometry of unidirectional rural parallel twin tunnels.

Table 1: Physical Parameters and Geometry of the Tunnels

| Parameters | Dimensions |
| :--- | :--- |
| Length of the Tunnels: | 8.67 Km |
| Number of Tunnel Bore: | 2 tunnels |
| Number of Lanes per Tunnel: | 4 lanes |
| Maximum Tunnel Altitude above sea level (@ East Portal): | 700 m |
| Tunnel Slope / Gradient: | $\pm 2.0 \%$ |
| Tunnel Cross-sectional Area: | 202 sqm |
| Tunnel Perimeter: | 60 m |
| Maximum Height at the Tunnel Crown: | 9.5 m |
| Tunnel Width at the Pavement Level: | 23.5 m |

### 2.1.2 Traffic Input Data

Table 2: Traffic Profile and Density and Designed Vehicle Speed

| Parameters | Dimensions |
| :--- | :--- |
| Annual Average Daily Traffic (AADT) | 65,000 vehicles/day |
| Peak Hour Traffic Volume @, 10\% of AADT | 6,500 vehicles/day |
| Therefore, Peak Hour Traffic Volume per Tunnel | 3,250 vehicles/hour/tunnel |
| Henceforth, Peak Hour Traffic Volume per Lane | 813 vehicles/hour/lane |
| Maximum Design Speed of the Vehicles | $130 \mathrm{Km} / \mathrm{hour}$ |

Table-3: Traffic Composition

| Category: | $\begin{aligned} & \frac{\text { Passenger }}{\text { Cars (PC) }} \\ & \hline \end{aligned}$ |  | Light Commercial <br> Vehicles (LCV) |  |  | Heavy Goods Vehicles (HGV) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Type: | Car | Taxi | LMV | LCV | $\begin{gathered} \hline \text { Mini } \\ \text { Bus } \\ \hline \end{gathered}$ | Std. <br> Bus | $\begin{gathered} 2 \\ \text { Axle } \end{gathered}$ | $\begin{gathered} 3 \\ \text { Axle } \end{gathered}$ | $\begin{gathered} \text { MAV (4-6 } \\ \text { Axle) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { MAV (>6 } \\ \text { Axle) } \end{gathered}$ |
| Percentage: | 39\% | 15\% | 5\% | 10\% | 2\% | 5\% | 5\% | 4\% | 10\% | 5\% |
| Fuel Type Composition: |  |  |  |  |  |  |  |  |  |  |
| Gasoline: | 23\% |  | 1\% |  |  | - |  |  |  |  |
| Diesel: | 31\% |  | 16\% |  |  | 29\% |  |  |  |  |

### 2.1.3 Traffic Output Profile: Calculated [1] for the Peak Hour Traffic with the Data of Table-1, 2 \& 3 above

Table 4: Fleet Composition in Both Tunnel at Different Traffic Vehicular Speed [for Down-Hill / Up-Hill Tubes]

| Vehicle Speed <br> (Km/hour) | No. of Vehicles in <br> Tunnels | No. of Cars |  |  | No. of LCV |  |  | No. of HGV |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Gasoline | Diesel | Gasoline | Diesel | 15 t | 23 t | 32 t |  |  |  |
| 0 | 3292 | 764 | 1013 | 22 | 537 | 165 | 296 | 494 |  |  |
| 10 | 1536 | 357 | 473 | 10 | 251 | 77 | 138 | 230 |  |  |
| 20 | 1409 | 327 | 434 | 10 | 230 | 70 | 127 | 211 |  |  |
| 30 | 939 | 218 | 289 | 6 | 153 | 47 | 85 | 141 |  |  |
| 40 | 704 | 164 | 217 | 5 | 115 | 35 | 63 | 106 |  |  |
| 50 | 564 | 131 | 173 | 4 | 92 | 28 | 51 | 85 |  |  |
| 60 | 470 | 109 | 145 | 3 | 77 | 23 | 42 | 70 |  |  |
| 70 | 403 | 93 | 124 | 3 | 66 | 20 | 36 | 60 |  |  |


| 80 | 352 | 82 | 108 | 2 | 57 | 18 | 32 | 53 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 90 | 313 | 73 | 96 | 2 | 51 | 16 | 28 | 47 |
| 100 | 282 | 65 | 87 | 2 | 46 | 14 | 25 | 42 |
| 110 | 256 | 59 | 79 | 2 | 42 | 13 | 23 | 38 |
| 120 | 235 | 55 | 72 | 2 | 38 | 12 | 21 | 35 |
| 130 | 217 | 50 | 67 | 1 | 35 | 11 | 20 | 33 |

### 2.1.4 Basis of Emission criteria being considered for estimation of the vehicular emission and pollution dilution by standard approach [1].

Table-5: Design Emission Criteria

| Description | $\underline{\text { Data }}$ |
| :--- | :---: |
| Base Year for the Emission Rates: | 2018 |
| Technology Standard Group / Class: | C |
| Design Year: | 2030 |
| Corresponding Time Shift Applicable: | 10 years |
| Therefore, Year of Base Emission Rates for Class C: | 2020 |

Table 6: Design Threshold Values for Emissions

| Pollutants | Design Parameters | Equivalent in g/m |
| :--- | :--- | ---: |
| Carbon Monoxide, CO (ambient) | 5 ppm | $5.716 \mathrm{mg} / \mathrm{m}^{3}$ |
| Carbon Monoxide, CO (admissible) | 70 ppm | $80.031 \mathrm{mg} / \mathrm{m}^{3}$ |
| Nitrogen Dioxide, NO2 (ambient) | 0.1 ppm | $0.188 \mathrm{mg} / \mathrm{m}^{3}$ |
| Nitrogen Dioxide, NO2 (admissible) | 1 ppm | $1.878 \mathrm{mg} / \mathrm{m}^{3}$ |
| Percentage of NO2 in NOx | $20 \%$ |  |
| Extinction Coefficient (Admissible), K (admissible) | $0.005 \mathrm{~m}^{-1}$ |  |
| Extinction Coefficient (Ambient), K (ambient) | $0.000 \mathrm{~m}^{-1}$ |  |

### 2.2 Fresh Air Flow Rate Demand and Visibility Condition During Dense Foggy Weather

### 2.2.1 Determination of fresh air demand for standard operation

Fresh Air Flow Rate Demand Calculated [1] for the Peak Hour Traffic at every $10 \mathrm{~km} / \mathrm{hour}$ intervals from standstill traffic ( $0 \mathrm{Km} /$ hour) to maximum design vehicular speed of 130 $\mathrm{km} / \mathrm{hour}$, for both down-hill and up-hill tunnels by adopting standard approach for the emission estimation [1] along with the Data of Table- $4,5 \& 6$ above.

Table 7 \& 8: Fresh Air Flow Rate Demand for Down-Hill Tunnel \& Up-Hill Tunnel

| $\begin{aligned} & \text { Vehicle } \\ & \text { Speed } \\ & (\mathrm{Km} / \text { hour }) \end{aligned}$ | Table-7: Down-Hill Tunnel |  |  |  | Table-8: Up-Hill Tunnel |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\underset{\left(\mathrm{m}^{3} / \mathrm{s}\right)}{\mathrm{CO}}$ | $\begin{aligned} & \mathrm{NO}_{2} \\ & \left(\mathrm{~m}^{3} / \mathrm{s}\right) \end{aligned}$ | Opacity ( $\mathrm{m}^{3} / \mathrm{s}$ ) | Max <br> Air <br> Demand $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | $\underset{\left(\mathrm{m}^{3} / \mathrm{s}\right)}{\mathrm{CO}}$ | $\begin{aligned} & \mathrm{NO} 2 \\ & \left(\mathrm{~m}^{3} / \mathrm{s}\right) \end{aligned}$ | Opacity ( $\mathrm{m}^{3} / \mathrm{s}$ ) | Max Air Demand $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ |
| 0 | 29 | 538 | 130 | 538 | 29 | 538 | 130 | 538 |
| 10 | 44 | 1077 | 257 | 1077 | 59 | 1342 | 311 | 1342 |
| 20 | 45 | 1006 | 295 | 1006 | 72 | 1378 | 359 | 1378 |
| 30 | 29 | 680 | 236 | 680 | 48 | 1003 | 302 | 1003 |
| 40 | 23 | 486 | 210 | 486 | 44 | 871 | 285 | 871 |
| 50 | 20 | 371 | 193 | 371 | 39 | 767 | 271 | 767 |
| 60 | 16 | 312 | 182 | 312 | 38 | 869 | 268 | 869 |
| 70 | 16 | 273 | 177 | 273 | 41 | 950 | 273 | 950 |
| 80 | 15 | 250 | 177 | 250 | 43 | 1053 | 280 | 1053 |


| 90 | 14 | 250 | 174 | 250 | 44 | 1033 | 284 | 1033 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 100 | 15 | 248 | 177 | 248 | 45 | 1021 | 282 | 1021 |
| 110 | 17 | 282 | 179 | 282 | 56 | 1051 | 282 | 1051 |
| 120 | 21 | 327 | 184 | 327 | 75 | 1080 | 283 | 1080 |
| 130 | 32 | 388 | 190 | 388 | 117 | 1142 | 290 | 1142 |
| Max FA <br> Demand | @ Down-Hill Tunnel |  |  | $1077 \mathrm{~m}^{3} / \mathrm{s}$ @ $10 \mathrm{Km} / \mathrm{h}$ due to $\mathrm{NO}_{2}$ | @ Up-Hill Tunnel: |  |  | $\begin{gathered} 1378 \mathrm{~m}^{3} / \mathrm{s} \\ @ 20 \mathrm{Km} / \mathrm{h} \text { due } \end{gathered}$ $\text { to } \mathrm{NO}_{2}$ |

### 2.2.2 Fresh air demand with respect to foggy weather situations

Also, to evaluate the visibility situations due to extreme foggy weather conditions during monsoon season, fresh air flow rate demand has been calculated for the same peak hour traffic at various vehicular speeds as in sl. no. 2.2.1 above, with enhanced visibility extinction coefficient values - ranging from design value of $0.005 \mathrm{~m}^{-1}$ up to $0.001 \mathrm{~m}^{-1}$. The detailed effects on the fresh air demand at various extinction coefficient values has been summarized in Table-9 below. It has been observed that the maximum fresh air demand requirement in both the down-hill and up-hill tunnels has little or no changes up to extinction coefficient value of $0.002 \mathrm{~m}^{-1}$. However, at extinction coefficient value of $0.001 \mathrm{~m}^{-1}$ there is a significant change in the maximum fresh air demand requirement in both the down-hill tunnel (increases by $37 \%$ ) and up-hill tunnel (increases by $30 \%$ ) than that of with the design extinction coefficient value of $0.005 \mathrm{~m}^{-1}$. The effects due to this aspect has been analyzed, for various factors having potential toward affecting visibility inside the tunnel, with detailed study on the optimal design length of light beam, in the subsequent clauses 2.2.3 to 2.2.6, below.

Table 9: Fresh Air Flow Rate Demand Summary for Multiple Extinction Coefficients to Evaluate the Visibility Situations due to Extreme Foggy Weather during Monsoon for Normal Mode Ventilation

| Fresh Air Demand with Design Extinction Coefficient: |  |  | $0.005 \mathrm{~m}^{-1}$ |  | $0.003 \mathrm{~m}^{-1}$ |  | $0.002 \mathrm{~m}^{-1}$ |  | $0.001 \mathrm{~m}^{-1}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Vehicle Speed (Km/hour) | $\begin{gathered} * \mathrm{CO} \\ \left(\mathrm{~m}^{3} / \mathrm{s}\right) \end{gathered}$ | $\begin{gathered} \begin{array}{c} \mathrm{NO}_{2} \\ \left(\mathrm{~m}^{3} / \mathrm{s}\right) \end{array} \end{gathered}$ | Opacity ( $\mathrm{m}^{3} / \mathrm{s}$ ) | Max Air Demand $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | Opacity ( $\mathrm{m}^{3} / \mathrm{s}$ ) | Max Air Demand $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | Opacity ( $\mathrm{m}^{3 / \mathrm{s}}$ ) | Max Air Demand ( $\mathrm{m}^{3} / \mathrm{s}$ ) | Opacity ( $\mathrm{m}^{3} / \mathrm{s}$ ) | Max Air Demand $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ |
| Down-Hill (LHS) Tunnel |  |  |  |  |  |  |  |  |  |  |
| 0 | 29 | 538 | 130 | 538 | 217 | 538 | 325 | 538 | 651 | 651 |
| 10 | 44 | 1077 | 257 | 1077 | 428 | 1077 | 642 | 1077 | 1283 | 1283 |
| 20 | 45 | 1006 | 295 | 1006 | 492 | 1006 | 738 | 1006 | 1477 | 1477 |
| 30 | 29 | 680 | 236 | 680 | 394 | 680 | 591 | 680 | 1182 | 1182 |
| 40 | 23 | 486 | 210 | 486 | 349 | 486 | 524 | 524 | 1048 | 1048 |
| 50 | 20 | 371 | 193 | 371 | 322 | 371 | 483 | 483 | 966 | 966 |
| 60 | 16 | 312 | 182 | 312 | 303 | 312 | 454 | 454 | 908 | 908 |
| 70 | 16 | 273 | 177 | 273 | 296 | 296 | 444 | 444 | 887 | 887 |
| 80 | 15 | 250 | 177 | 250 | 295 | 295 | 442 | 442 | 884 | 884 |
| 90 | 140 | 250 | 174 | 250 | 290 | 290 | 435 | 435 | 871 | 871 |
| 100 | 15 | 248 | 177 | 248 | 294 | 294 | 442 | 442 | 883 | 883 |
| 110 | 17 | 282 | 179 | 282 | 298 | 298 | 446 | 446 | 893 | 893 |
| 120 | 21 | 327 | 184 | 327 | 307 | 327 | 461 | 461 | 921 | 921 |
| 130 | 32 | 388 | 190 | 388 | 316 | 388 | 474 | 474 | 949 | 949 |
| Max Fresh Air Demand @ Down-Hill Tunnel: |  |  | $1077 \mathrm{~m}^{3} / \mathrm{s}$ <br> @ $10 \mathrm{Km} / \mathrm{h}$ due to $\mathrm{NO}_{2}$ |  | $1077 \mathrm{~m}^{3} / \mathrm{s}$@ $10 \mathrm{Km} / \mathrm{h}$ due to |  | $1077 \mathrm{~m}^{3} / \mathrm{s}$ <br> @ $10 \mathrm{Km} / \mathrm{h}$ due to $\mathrm{NO}_{2}$ |  | $1477 \mathrm{~m}^{3} / \mathrm{s}$ <br> (a) $20 \mathrm{Km} / \mathrm{h}$ due to Opacity |  |
| Up-Hill (RHS) Tunnel |  |  |  |  |  |  |  |  |  |  |
| 0 | 29 | 538 | 130 | 538 | 217 | 538 | 325 | 538 | 651 | 651 |
| 10 | 59 | 1342 | 311 | 1342 | 518 | 1342 | 777 | 1342 | 1553 | 1553 |
| 20 | 72 | 1378 | 359 | 1378 | 598 | 1378 | 896 | 1378 | 1793 | 1793 |
| 30 | 48 | 1003 | 302 | 1003 | 503 | 1003 | 755 | 1003 | 1509 | 1509 |
| 40 | 44 | 871 | 285 | 871 | 476 | 871 | 714 | 871 | 1427 | 1427 |
| 50 | 39 | 767 | 271 | 767 | 452 | 767 | 678 | 767 | 1356 | 1356 |
| 60 | 38 | 869 | 268 | 869 | 447 | 869 | 671 | 869 | 1341 | 1341 |


| 70 | 41 | 950 | 273 | 950 | 454 | 950 | 681 | 950 | 1363 | 1363 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 80 | 43 | 1053 | 280 | 1053 | 467 | 1053 | 701 | 1053 | 1402 | 1402 |
| 90 | 44 | 1033 | 284 | 1033 | 474 | 1033 | 711 | 1033 | 1421 | 1421 |
| 100 | 45 | 1021 | 282 | 1021 | 470 | 1021 | 705 | 1021 | 1411 | 1411 |
| 110 | 56 | 1051 | 282 | 1051 | 470 | 1051 | 704 | 1051 | 1409 | 1409 |
| 120 | 75 | 1080 | 283 | 1080 | 472 | 1080 | 707 | 1080 | 1415 | 1415 |
| 130 | 117 | 1142 | 290 | 1142 | 483 | 1142 | 725 | 1142 | 1450 | 1450 |
| Max Fresh Air Demand @ Up-Hill Tunnel: |  |  | $1378 \mathrm{~m}^{3} / \mathrm{s}$ <br> @ $20 \mathrm{Km} / \mathrm{h}$ due to $\mathrm{NO}_{2}$ |  | $1378 \mathrm{~m}^{3} / \mathrm{s}$ <br> (a) $20 \mathrm{Km} / \mathrm{h}$ due to $\mathrm{NO}_{2}$ |  | $1378 \mathrm{~m}^{3} / \mathrm{s}$ <br> (a) $20 \mathrm{Km} / \mathrm{h}$ due to $\mathrm{NO}_{2}$ |  | $1793 \mathrm{~m}^{3} / \mathrm{s}$ <br> (a) $20 \mathrm{Km} / \mathrm{h}$ due to Opacity |  |

### 2.2.3 Determination of Extinction Coefficient

Comparative study of the extinction coefficient with respect to percentage of intensity of the light at the receiver vis-s-vis intensity of the light source, according to extinction coefficient expressed by equation (1), below.

$$
\begin{equation*}
K=-\frac{1}{L} \cdot \ln \left\{\frac{I}{I o}\right\} \tag{1}
\end{equation*}
$$

$L=$ Beam length between source and receiver, $I o=$ Intensity of the Light Source, $I=$ Intensity of the Light at the Receiver

### 2.2.4 Dense Fog Analysis

- The mass concentration of $\mathrm{PM}_{2.5}$ ( $\mu \mathrm{PM} 2.5$ ) is ranged from $121-375 \mu \mathrm{~g} / \mathrm{m}^{3}$, and the interaction between fog droplets and fine particles is analyzed [3].
- And with the equation 5 [1]: Extinction Coefficient, $K=f_{\text {vis. }}$. PM2. 5 Where, f vis is a conversion factor $=0.0047 \mathrm{~m}^{2} / \mathrm{mg}$ $\mu_{\text {PM } 2.5}=375 \mu \mathrm{~g} / \mathrm{m}^{3}+15 \%$ (in excess for more safety) $=430 \mu \mathrm{~g} / \mathrm{m}^{3}$
- Therefore, due to dense fog in the atmosphere the Extinction Coefficient at ambient is, $K_{a m b}=0.002 \mathrm{~m}^{-1}$

Table 10: Comparative of Visibility Condition \& Fog Analysis Visibility Condition vis-à-vis Extinction Coefficient

| Parameters | Design <br> Criteria | Comparisons of Visibility Condition |  |  |  |  |  | Fog Analysis Visibility Condition |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Extinction Coefficient (Admissible), $\mathrm{K}_{\mathrm{adm}}\left(\mathrm{m}^{-1}\right)$ | 0.005 | 0.003 |  | 0.002 |  | 0.001 |  | 0.005 |  | 0.007 |
| Extinction Coefficient (Ambient), $\mathrm{K}_{\mathrm{amb}}\left(\mathrm{m}^{-1}\right.$ ) | 0 | 0 |  | 0 |  | 0 |  | 0.002 |  | 0.002 |
| Extinction Coefficient (Difference), $\mathrm{K}=\mathrm{K}_{\mathrm{adm}}-\mathrm{K}_{\mathrm{amb}}\left(\mathrm{~m}^{-1}\right)$ | 0.005 | 0.003 |  | 0.002 |  | 0.001 |  | 0.003 |  | 0.005 |
| Percentage of Intensity of the Light at the Receiver (I) w.r.t. Source (Io), I/Io (\%) | 20\% | 20\% | 38\% | 20\% | 52\% | 20\% | 72\% | 20\% | 38\% | 20\% |
| Length of Light Beam, L <br> (m) | 322 | 536 | 322 | 805 | 322 | 1209 | 322 | 536 | 322 | 322 |
| Remarks / Observations | More Light beam length @ design I/Io ratio \&Better I/Io ratio @ design Light beam length |  |  |  |  |  |  |  |  |  |

### 2.2.5 Impact on Fresh air demand vis-à-vis extinction coefficient

The comparative study of Table-10 vis-à-vis Table-9 above reflects that by enhancing the extinction coefficient, resulting considerable enhancement of either the length of light beam or the percentage of intensity of the light at the receiver, the fresh air demand has very little or no changes up to an extinction coefficient value of $0.002 \mathrm{~m}^{-1}$. Therefore, preliminary review and study could be conclusive that during extreme foggy weather conditions in the region the estimated fresh air demand can meet the visibility parameters comfortably up to an extinction coefficient value of $0.002 \mathrm{~m}^{-1}$ (i.e., having at least 805 m long light beam with $20 \%$ intensity [1] of the light at the receiver or safe design light beam length of 322 m with $52 \%$ intensity of the light at the receiver) without any modification in the normal ventilation design criteria adopted, even though quantum of fresh air requirement increases for pollution dilution due to opacity, by considering enhancing admissible extinction coefficient and zero ambient extinction coefficient. But, the fresh air requirement for dilution of $\mathrm{NO}_{2}$ still governs here in the design criteria of this particular case study, except for certain higher vehicular speed at enhanced extinction coefficient in the down-hill tunnel where fresh air demand is governed due to opacity dilutions. Nevertheless, these have an insignificant impact in the design as the ventilation system been designed for maximum fresh air demand requirement at congested slow traffic movement.

Furthermore, the results of the extinction coefficients with dense fog analysis in Table-10 above indicates that with $\mathrm{K}=0.003 \mathrm{~m}^{-1}$ or $0.005 \mathrm{~m}^{-1}$ it shall still be within the acceptable limit of visibility (i.e., having at least 536 m long light beam with $20 \%$ intensity of the light at the receiver or safe design light beam length of 322 m with $38 \%$ intensity of the light at the receiver or at least 322 m long light beam with $20 \%$ intensity of the light at the receiver even when considering in the design with hazy extinction coefficient of $0.007 \mathrm{~m}^{-1}$ prevailing inside the tunnel) without any modification in the normal ventilation design criteria adopted, even though quantum of fresh air requirement increases with decreasing differences of extinction coefficient between admissible and ambient for pollution dilution due to opacity, but the fresh air requirement for dilution of $\mathrm{NO}_{2}$ still governs here in the design criteria of this particular case study.

### 2.2.6 Visibility Analysis for a Safe Stopping Sight Distance

Safe Stopping Sight Distance (SSSD) is the distance required for a driver to bring the vehicle to a stop after observing any object on the road. It is calculated based on the design speed of the road and the reaction time of the driver. Following formulas [4] are adopted to calculate the SSSD:

$$
S S S D=L D+B D
$$

$\mathrm{LD}=\mathrm{Lag}$ or Reaction Distance $=V . t R$
$\mathrm{BD}=$ Braking Distance $=V^{2} \cdot\left[\frac{1}{2 . g . f+s}\right]$
$V=$ Design Vehicle Speed in metre per second
$t R=$ Reaction Time in seconds
$g=$ Acceleration due to Gravity $=9.81$ metre $/$ second $^{2}$
$f=$ Coefficient of Longitudinal Friction $=0.35 \sim 0.40$ [5]
$s=$ gradient in $\%$

Table 11: Effects of Visibility Condition due to Safe Stopping Sight Distance (SSSD)

| Design Vehicle <br> Speed, V (Km/h) | 10 | 20 | 30 | 40 | 50 | 60 | 70 | 80 | 90 | 100 | 110 | 120 | 130 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Safe Stopping Sight <br> Distance, SSSD (m) | 8 | 19 | 32 | 47 | 65 | 85 | 107 | 132 | 159 | 189 | 221 | 255 | 292 |
| Light Level (i.e., I/Io <br> in \%) @ Extinction <br> Coefficient, K $=$ <br> $0.005 \mathrm{~m}^{-1}$ | $96 \%$ | $91 \%$ | $85 \%$ | $79 \%$ | $72 \%$ | $66 \%$ | $59 \%$ | $52 \%$ | $45 \%$ | $39 \%$ | $33 \%$ | $28 \%$ | $23 \%$ |

Note: Therefore, Table-11 vis-à-vis Table-10 above concludes that since the Length of Light Beam, L = 322 m [@ Minimum Acceptable Visibility or Light Level (i.e., I/Io $=20 \%$ ) and Design Extinction Coefficient, $\mathrm{K}=0.005 \mathrm{~m}^{-1}$ ] is Greater than SSSD as well as I/Io Ratio at all Design Speed, the adopted Design Basis is safe and holds good.

### 2.3 Jet Fan Calculation Procedure for Longitudinal Ventilation

### 2.3.1 Total Pressure and Thrust in the Tunnel

To determine the quantities / number of jet fans required for the longitudinal road tunnel ventilation system it is pertinent to determine the total thrust required to overcome the gross total pressure drops / losses in the tunnel [6].

The total pressure loss and thrust in the tunnel defined by the following equations:

$$
\begin{align*}
& \Delta P_{\text {Total }}=\Delta \mathrm{P}_{\text {ent }}+\Delta \mathrm{P}_{\text {exit }}+\Delta \mathrm{P}_{\mathrm{wf}}+\Delta \mathrm{P}_{\mathrm{w}}+\Delta \mathrm{P}_{\text {veh }}+\Delta \mathrm{P}_{\text {ce }}+\Delta \mathrm{P}_{\text {fire }}+\Delta \mathrm{P}_{\text {met }}  \tag{1}\\
& \mathrm{T}_{\text {Total }}=\Delta \mathrm{P}_{\text {Total }} \times \mathrm{A}_{\text {Tunnel }}
\end{align*}
$$

$\Delta P_{\text {Total }}=$ total pressure drops (in $\mathbf{P a}$ )
$\mathrm{T}_{\text {Total }}=$ total thrust required (in $\mathbf{N}$ )
ATunnel $=$ tunnel cross-sectional area (in $\mathbf{m}^{2}$ )
$\Delta \mathrm{P}_{\text {ent }} \& \Delta \mathrm{P}_{\text {exit }}=$ pressure drops due to tunnel entrance and exit
$\Delta \mathrm{P}_{\mathrm{wf}}=$ pressure drops due to tunnel wall friction
$\Delta \mathrm{P}_{\mathrm{w}}=$ pressure drops due to adverse wind
$\Delta \mathrm{P}_{\text {veh }}=$ pressure drops $/$ gains due to vehicles / piston effects
$\Delta \mathrm{P}_{\text {ce }}=$ pressure drops due to chimney effect / fire buoyancy
$\Delta \mathrm{P}_{\text {fire }}=$ pressure drops due to fire blockage (in fire scenario only)
$\Delta \mathrm{P}_{\text {met }}=$ pressure drops due to meteorological conditions

### 2.3.2 Jet Fan Estimation

Number of operating jet fans required for the longitudinal road tunnel ventilation system [6] is calculated by the following equations:
$\mathrm{N}_{\text {Jet Fan }}=\left\{\mathrm{T}_{\text {Total }}\right\} \div\left\{\mathrm{T}_{\text {Jet Fan }} \times\left(\eta_{\mathrm{i}} \times \eta_{\mathrm{v}} \times \eta_{\rho}\right)\right\}$
$\mathrm{N}_{\text {Jet Fan }}=$ number of operating jet fans
$\mathrm{T}_{\text {Total }}=$ total thrust required (in $\mathbf{N}$ )
$\mathrm{T}_{\text {Jet Fan }}=$ nominal jet fan thrust (in $\mathbf{N}$ )
$\eta_{i}=$ installation efficiency
$\eta_{\mathrm{v}}=$ velocity derating factor
$\eta_{\rho}=$ density derating factor

### 2.3.3 Longitudinal Tunnel Ventilation Summary

With the above equations in clauses $2.3 .1 \& 2.3 .2$ the results [6] are summarised and tabulated below in Table- 12 for the fresh air flow rate demand and visibility condition during dense Foggy weather, as designed at clause 2.2 above, for normal mode tunnel ventilation, as well as for the fire mode ventilation.

Table 12: Battery of Jet Fans for Longitudinal Tunnel Ventilation

| Ventilation Parameters | Normal Mode |  | Emergency (Fire) Mode |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Down-Hill Tunnel | Up-Hill Tunnel | Down-Hill Tunnel | Up-Hill Tunnel |
| Maximum Fresh Air Demand (m/s ${ }^{3}$ ) @ Dense Foggy Weather with $0.002 \mathrm{~m}^{-1}$ Ext. Coeff. | 1077 | 1378 | - | - |
| Critical Velocity @ Fire size of 200 MW (m/s) | - | - | 3.30 | 3.55 |
| Total Thrust Required (N) | 141058 | 198507 | 131230 | 133287 |
| Selected Jet Fan Thrust (N) | 2200 | 2200 | 2200 | 2200 |
| Installation Efficiency, $\eta_{\mathrm{i}}=$ | 0.8 | 0.8 | 0.8 | 0.8 |
| Velocity Derating Factor, $\eta_{\mathrm{v}}=$ | 0.83 | 0.79 | 0.90 | 0.89 |
| Density Derating Factor, $\eta_{\rho}=$ | 0.87 | 0.87 | 0.79 | 0.82 |
| Total Number of Operating Jet Fans | 111 | 165 | 105 | 104 |
| Number of Jet Fans mounted in each location | 3 | 3 | 3 | 3 |
| Max equal distance between Jet Fans sets (m) | 228 | 155 | 241 | 243 |
| Min recommended distance between Jet Fans sets [7] is 10 x tunnel hydraulic diameter (m) | 135 | 135 | 135 | 135 |

### 2.3.4 Smoke Management Consideration

In the above Table-12 vis-à-vis explanations given in the above clauses 2.2.2, 2.2.5 \& note @ 2.2.6, along-with 1-D Simulation [6] with a Fire size of 200 MW HRR, the number of Jet Fans batteries, required for a design criteria and dense foggy condition for a normal mode longitudinal ventilation system, shall be sufficient for an effective smoke management at the worst probable locations of the fire scenarios. Furthermore, the recommended distance between jet fan sets [7][8] and the economics of the optimization on the design for the installation of these jet fans near the portals [9] are also adhered to.

## 3. SUMMARY AND CONCLUSION

The above analysis and study with detailed calculations of the longitudinal mechanical ventilation system designed, with batteries of jet fans mounted at the crown of the tunnels, for normal mode operation reflects that even during extreme foggy weather condition the basic acceptable limit of visibility will be achievable without any modification to the system for the selected normal mode ventilation basic design criterion.

Even though the quantum of fresh air requirement increases with decreasing differences of extinction coefficient between admissible and ambient for pollution dilution due to opacity, the fresh air requirement for dilution of $\mathrm{NO}_{2}$ still governs here in the design because of a very high standard of international design criterion being adopted for nitrogen dioxide.

Furthermore, the longitudinal mechanical ventilation system for normal mode shall also cater to an effective smoke management, up to a fire size of 200 MW HRR, at any probable locations of the fire scenarios, with the selected jet fans specifications and quantities.

It can be concluded that the adopted design is optimized with a very reliable functional requirement achievability for all weather conditions and stringent safety standards.

## 4. REFERENCES

[1] PIARC 2019R02EN - Road Tunnels: Vehicle Emissions and Air Demand for Ventilation
[2] RVS 09.02.32 June 2010 - Tunnel Equipment Ventilation Systems Fresh Air Demand: Air Demand Calculation
[3] Paper: "Fog Droplet Size Distribution and the Interaction between Fog Droplets and Fine Particles during Dense Fog in Tianjin, China" - by Qing Liu, Bingui Wu, Zhaoyu Wang and Tianyi Hao [Read full-text or Download full-text PDF at: https://www.researchgate.net/publication/339761583 Fog Droplet Size Distribution and the I nteraction_between_Fog_Droplets_and_Fine_Particles_during_Dense_Fog_in_Tianjin_China]
[4] Paper: "Sight Distances: Lecture Notes in Transportation Systems Engineering" - by Prof Tom V. Mathew [https://www.civil.iitb.ac.in/tvm/nptel/303 SigDst/web/web.html]
[5] Highway Engineering Design Data Hand Book (Geometric Design \& Pavement Design) Compiled by Dr. P. Nanjundaswamy / IRC
[6] Detailed calculation procedures, 1-D simulation and results are incorporated under a separate design head / volume of this paper. Not able to submit as a part of this paper because it is quite extensive and voluminous to restrict within the stipulation of not exceeding 8 pages. However, if permitted, I can include the same in this paper or submit separately.
[7] PIARC 05.02.B - 1995: Vehicles Emissions Air Demand Environment Longitudinal Ventilation: Article IV.2.1 (c) @ page-51 - Longitudinal distance ( $\eta_{3}$ )
[8] 2011 ASHRAE Handbook - HVAC Applications: Chapter-15: Enclosed Vehicular Facilities: Equipment - Fans - Number \& Sizes of Fans @ 15.33
[9] PIARC 05.05.B - 1999: Fire and Smoke Control in Road Tunnels: Article V.1.2.1; last para @ page-145 - Longitudinal system

